

Designation: A 934/A 934M - 04

Standard Specification for Epoxy-Coated Prefabricated Steel Reinforcing Bars¹

This standard is issued under the fixed designation A 934/A 934M; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon (ϵ) indicates an editorial change since the last revision or reapproval.

1. Scope*

- 1.1 This specification covers deformed and plain steel reinforcing bars which prior to surface preparation are prefabricated and then coated with a protective fusion-bonded epoxy coating by electrostatic spray or other suitable method.
- 1.2 Prefabricated steel reinforcing bars coated with fusionbonded epoxy formulations in accordance with this specification are not intended to be bent or rebent.

Note 1—Experience has shown epoxy-coated prefabricated steel reinforcing bars have limited capability to be successfully field bent or rebent (see 9.3.1). If the construction project requires epoxy-coated steel reinforcing bars that must be able to sustain field bending or rebending, the use of coated bars conforming to Specification A 775/A 775M should be considered.

- 1.3 Organic coatings other than epoxy may be used provided they meet the requirements of this specification.
- 1.4 Requirements for epoxy coatings are contained in Annex A1.
- 1.5 Guidelines for application process and product test procedures of epoxy coatings for steel reinforcing bars are presented in Appendix X1.
- 1.6 Guidelines for construction practices at the job-site for coated steel reinforcing bars are presented in Appendix X2.
- 1.7 This specification is applicable for orders in either SI (metric) units (as Specification A 934M) or inch-pound units [as Specification A 934].
- 1.8 The values stated in either SI or inch-pound units are to be regarded as standard. Within the text, the inch-pound units are shown in brackets. The values stated in the two systems are not exact equivalents: therefore, each system must be used independently of the other. Combining values from the systems may result in nonconformance with this specification.
- 1.9 This standard does not purport to address all of the safety concerns, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety and health practices and determine the applicability of regulatory limitations prior to use.

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2. Referenced Documents

- 2.1 ASTM Standards: ²
- A 615/A 615M Specification for Deformed and Plain Billet-Steel Bars for Concrete Reinforcement
- A 706/A 706M Specification for Low-Alloy Steel Deformed and Plain Bars for Concrete Reinforcement
- A 775/A 775M Specification for Epoxy-Coated Steel Reinforcing Bars
- A 944 Test Method for Comparing Bond Strength of Steel Reinforcing Bars to Concrete Using Beam-End Specimens
- A 996/A 996M Specification for Rail-Steel and Axle-Steel Deformed Bars for Concrete Reinforcement
- B 117 Practice for Operating Salt Spray (Fog) Testing Apparatus
- D 4060 Test Method for Abrasion Resistance of Organic Coatings by the Taber Abraser
- G 8 Test Methods for Cathodic Disbonding of Pipeline Coatings
- G 14 Test Method for Impact Resistance of Pipeline Coatings (Falling Weight Test)
- G 20 Test Method for Chemical Resistance of Pipeline Coatings
- G 42 Test Method for Cathodic Disbonding of Pipeline Coatings Subjected to Elevated Temperatures
- G 62 Test Methods for Holiday Detection in Pipeline Coatings
- 2.2 NACE International Standard:³
- RP 0287 Field Measurement of Surface Profile of Abrasive Blast Cleaned Steel Surfaces Using a Replica Tape
- 2.3 Society for Protective Coatings Specifications:⁴
- SSPC-SP 10 Near-White Blast Cleaning
- SSPC-VIS 1 Pictorial Surface Preparation Standards for Painting Steel Surfaces
- SSPC-PA 2 Measurement of Dry Coating Thickness with Magnetic Gages

¹ This specification is under the jurisdiction of ASTM Committee A01 on Steel, Stainless Steel, and Related Alloys and is the direct responsibility of Subcommittee A01.05 on Steel Reinforcement.

² For referenced ASTM standards, visit the ASTM website, www.astm.org, or contact ASTM Customer Service at service@astm.org. For *Annual Book of ASTM Standards* volume information, refer to the standard's Document Summary page on the ASTM website.

³ Available from NACE International, 1440 South Creek, Houston, TX 77084.

⁴ Available from Society for Protective Coatings, 40 24th Street, Pittsburgh, PA

2.4 American Concrete Institute Specification:⁵

ACI 301 Specifications for Structural Concrete

ACI 315 Details and Detailing of Concrete Reinforcement

2.5 Concrete Reinforcing Steel Institute:⁶

"Voluntary Certification Program for Fusion-Bonded Epoxy Coating Applicator Plants"

3. Terminology

- 3.1 Definitions of Terms Specific to This Standard:
- 3.1.1 *conversion coating*—a preparation of the blast-cleaned metal surface prior to coating application that is designed to pretreat the metal to promote coating adhesion, reduce metal coating reactions, improve corrosion resistance, and increase blister resistance.
- 3.1.2 *disbonding*—loss of adhesion between the fusion-bonded epoxy coating and the steel reinforcing bar.
- 3.1.3 fusion-bonded epoxy coating—a product containing pigments, thermosetting epoxy resins, crosslinking agents, and other additives, which is applied in the form of a powder onto a clean, heated metallic substrate and fuses to form a continuous barrier coating.
- 3.1.4 *holiday*—a discontinuity in a coating that is not discernible to a person with normal or corrected vision.
- 3.1.5 *patching material*—a liquid two-part epoxy coating used to repair damaged areas.
- 3.1.6 *prefabricated bars*—steel reinforcing bars that are cut to specified lengths and bent to the required shapes prior to coating.
- 3.1.7 wetting agent—a material that lowers the surface tension of water allowing it to penetrate more effectively into small discontinuities in the coating, giving a more accurate indication of the holiday count.

4. Ordering Information

- 4.1 It shall be the responsibility of the purchaser to specify all requirements that are necessary for the coated steel reinforcing bars under this specification. Such requirements to be considered include, but are not limited to, the following:
 - 4.1.1 Reinforcing bar specification and year of issue,
 - 4.1.2 Quantity and bend types of bars,
 - 4.1.3 Size and grade of bars,
- 4.1.4 Requirements for the powder coating and provision of test data (5.2),
 - 4.1.5 Requirements for patching material (5.3),
 - 4.1.6 Quantity of patching material,
- 4.1.7 Coated specimens shall be provided by the manufacturer out of the same production lot of coated reinforcement being furnished to the purchaser (13.1).
- 4.1.8 Whether a report on tests performed on the coated steel reinforcing bars being furnished is required (15.1),
- 4.1.9 Method for temperature reduction of hot, freshly coated bars (unforced air cool down or forced air/water quench) (8.3).

Note 2—A typical ordering description is as follows: Deformed Grade 420 bars to ASTM A 615M – _____; 6000 m, No. 19; cut to specified lengths and fabricated prior to coating; epoxy-coated to ASTM A 934M – ____; including written certifications for the powder coating and coated bars and 1 L of patching material.

[Deformed Grade 60 bars to ASTM A 615 – _____; 20 000 ft, No. 6; cut to specified lengths and fabricated prior to coating; epoxy-coated to ASTM A 934 – ____; including written certifications for the powder coating and coated bars and 1 qt. of patching material.

5. Materials

5.1 Steel reinforcing bars to be coated shall meet the requirements of one of the following specifications: A 615M, A 706M, or A 996M [A 615, A 706, or A 996] as specified by the purchaser and shall be free of contaminants such as oil, grease, or paint.

Note 3—Prior to coating, the steel reinforcing bars should be inspected for their suitability for coating. Bars with sharp edges on the deformations, rolled-in slivers, or other surface imperfections are difficult to coat properly. The coating will flow away from the sharp edges and may result in inadequate coating thickness at these points.

- 5.2 The powder coating shall meet the requirements of Annex A1. Upon request, the purchaser shall be provided with test data for review.
- 5.2.1 A written certification shall be furnished to the purchaser that properly identifies the lot designation of the powder coating used in the order, material quantity represented, date of manufacture, name and address of the powder coating manufacturer, and a statement that the supplied powder coating is the same composition as that qualified according to Annex A1 of this specification. The powder coating shall be used within the powder coating manufacturer's written recommended shelf life.
- 5.2.2 If specified in the order, a representative 0.2-kg [8-oz] sample of the powder coating shall be supplied to the purchaser from each batch. The sample shall be packaged in an airtight container and identified by the batch designation.
- 5.2.3 The powder coating shall be maintained in a temperature-controlled environment following the written recommendations of the powder coating manufacturer until ready for use, at which point the powder coating shall be given sufficient time to reach approximate plant ambient temperature.
- 5.3 If specified in the order, patching material, compatible with the coating, inert in concrete, and recommended by the powder coating manufacturer, shall be supplied to the purchaser.

6. Prefabrication of Steel Reinforcing Bars

6.1 The steel reinforcing bars to be coated shall have been prefabricated in accordance with the purchaser's requirements or project specifications prior to surface preparation.

Note 4—Drive rolls on shear beds and backup barrels on benders should be protected with a suitable covering to minimize crushing or creating rollover damage to the steel reinforcing bar deformations during the fabrication process.

7. Surface Preparation of Steel Reinforcing Bars

7.1 The surface of the steel reinforcing bars shall be cleaned by abrasive blast steel grit to near-white metal in accordance with SSPC-SP10.

⁵ Available from American Concrete Institute, 38800 Country Club Drive, Farmington Hills, MI 48331.

⁶ Available from Concrete Reinforcing Steel Institute, 933 N. Plum Grove Rd., Schaumburg, IL 60173.

- 7.1.1 The final surface condition shall be defined according to SSPC-VIS1.
- 7.1.2 Average blast profile roughness depth readings of 37 to 100 μ m [1.5 to 4.0 mils], as determined by replica tape measurements using RP0287 or other methods acceptable to the purchaser, shall be considered suitable as an anchor pattern.

Note 5—The use of a profilometer type surface measurement instrument that measures the peak count as well as the maximum profile depth is recommended.

7.2 A steel grit of Rockwell Hardness C50 or higher shall be used. A maximum of 5 % steel shot is allowable in the media.

Note 6—Recycled steel grit abrasive should be maintained so as to minimize contaminants such as oil, salt, and dust caused by the blasting operation.

7.3 Multidirectional, high-pressure dry air knives shall be used after blast cleaning to remove dust, grit, and other foreign matter from the steel surface. The air knives shall not deposit oil on the steel reinforcing bars.

Note 7—It is recommended that incoming steel reinforcing bars and blast media be checked for salt contamination prior to use. Blast media found to be salt contaminated should be rejected. Steel reinforcing bars found to be salt contaminated from exposure to deicing salts or salt spray should be cleaned by acid washing or other suitable methods to remove salt contiminants from the surface prior to blast cleaning.

7.4 It shall be permissible for the manufacturer to use a chemical wash or conversion of the steel reinforcing bar surface, or both, to enhance coating adhesion. This pretreatment shall be applied after abrasive cleaning and before coating, in accordance with the written application instructions specified by the pretreatment manufacturer.

8. Coating Application

- 8.1 Cleaned, uncoated steel reinforcing bars shall be handled by personnel wearing clean gloves to prevent contamination of the steel surface.
- 8.2 If pretreatment is used in the preparation of the surface the powder coating shall be applied to the cleaned and pretreated steel reinforcing bar surface as soon as possible after surface treatments have been completed, and before visible oxidation of the surface occurs as discernible to a person with normal or corrected vision. In no case shall application of the coating be delayed more than 3 h after cleaning.
- 8.3 The fusion-bonded epoxy powder coating shall be applied in accordance with the written recommendations of the manufacturer of the powder coating for initial steel surface temperature range and post application cure requirements. The temperature of the surface immediately prior to coating shall be

measured using infrared guns or temperature-indicating crayons at least once every 30 min during continuous operations.

Note 8—The use of infrared and temperature-indicating crayon measurement of the steel reinforcing bars is recommended.

8.4 The powder coating shall be applied by electrostatic spray or other suitable method.

9. Requirements for Coated Steel Reinforcing Bars

9.1 Coating Thickness:

9.1.1 The coating thickness on straight sections of bars, after curing, shall be 175 to 300 μ m [7 to 12 mils]. The coating thickness on bent sections of bars, after curing, shall be 175 to 405 mm [7 to 16 mils]. Bent sections of bars are defined as the entire radius of each bend and portions of the bar extending 150 mm [6 in.] beyond the beginning and ending bend points on the bar. Bars that are bent with a radius such as Bend Type 9 (Fig. 1) shall have the same coating thickness requirements as straight bars, when the "H" dimension is equal to or less than one-half of the "R" dimension.

Note 9—Section 9.1.1 permits a maximum coating thickness of 405 μ m [16 mils] on bent sections of bars. Discussion of the effect of the permissible increased coating thickness on the anchorage capacity of bent bars embedded in concrete is presented in Appendix X3.

- 9.1.2 A single recorded steel reinforcing bar coating thickness measurement is the average of three individual gage readings obtained between three consecutive deformations. A minimum of five recorded measurements shall be taken approximately evenly spaced along each side of the test specimen (a minimum of ten recorded measurements per bar).
- 9.1.3 For acceptance purposes, the average of all recorded coating thickness measurements shall not be less than the specified minimum thickness or more than the specified maximum thickness. No single recorded coating thickness measurement shall be less than 80 % of the specified minimum thickness or more than 120 % of the specified maximum thickness.
- 9.1.4 Measurements shall be made in accordance with SSPC-PA2 following the instructions for calibration and use recommended by the thickness gage manufacturer. Pull-off or fixed probe gages shall be used. Pencil-type pull-off gages that require the operator to observe the readings at the instant the magnet is pulled from the surface shall not be used.
- 9.1.5 Approximately equal numbers of straight and bent sections of coated steel reinforcing bars shall be evaluated. On the bent sections, approximately half of these measurements shall be made on the inside radius and half on the outside radius.

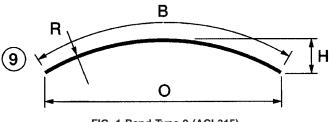


FIG. 1 Bend Type 9 (ACI 315)

9.2 Coating Continuity:

- 9.2.1 Holiday checks to determine the acceptability of the coated steel reinforcing bars prior to shipment shall be made at the manufacturer's plant with a hand-held 67.5-V, 80 000- Ω , wet-sponge-type dc holiday detector.
- 9.2.2 There shall not be on average more than three holidays per metre [one holiday per linear foot] on the coated steel reinforcing bars. The average applies to the full production length of a bar. For coated bars less than 0.3 m [1 ft] in length, the maximum shall be one holiday.
- 9.2.3 A wetting agent shall be used in accordance with Test Methods G 62 in the inspection for holidays on the coated steel reinforcing bars.
- 9.2.4 The coated steel reinforcing bars shall be inspected at a maximum rate of 0.05 m/s [0.2 ft/s], wiping one side of the coated bar and then the other with the wetted sponge or an equivalent method.

Note 10—Care should be taken that contact of the sponge along the entire steel surface being tested is maintained in order to obtain an accurate holiday count.

9.3 Coating Flexibility:

- 9.3.1 Coating flexibility shall be evaluated by bending production-coated steel reinforcing bars around a mandrel eight times the nominal diameter (*d*) of the bar at a uniform rate and within 5 s. The exception to the 8*d* mandrel requirement shall be Nos. 43 and 57 [Nos. 14 and 18] bars. A 430-mm [17-in.] mandrel (9.71*d*) shall be used when evaluating No. 43 [No. 14] bar, and a 580-mm [23-in.] mandrel (10.22*d*) shall be used with No. 57 [No. 18] bar. For Nos. 10 to 16 [Nos. 3 to 5] coated steel reinforcing bars, the bend angle after rebound shall be 9°. For No. 19 to 57 [Nos. 6 to 18] coated steel reinforcing bars, the bend angle after rebound shall be 6°. The two longitudinal deformations shall be placed in a plane perpendicular to the mandrel radius. The temperature of the test specimens shall be between 20 and 30°C [68 and 86°F].
- 9.3.2 Cracking or disbonding of the coating on the outside radius of the bent bar discernible to a person with normal or corrected vision shall be considered cause for rejection of the coated steel reinforcing bars represented by the bend test sample.

9.4 Coating Adhesion:

- 9.4.1 Coating adhesion shall be evaluated by cathodic disbondment testing of production-coated steel reinforcing bars. Test Method G 42 shall be followed except, (I) the cathode shall be a 200-mm [8-in.] long coated steel reinforcing bar; (2) the anode shall be a 150-mm [6-in.] long solid platinum electrode (1.6-mm [0.06-in.] nominal diameter) or platinized wire (3-mm [0.12-in.] nominal diameter); (I) a calomel reference electrode shall be used; (I) the electrolyte solution shall be 3 % NaCl by mass dissolved in distilled water; (I) the electrolyte solution temperature shall be 3 mm [0.12 in.] in diameter; (I) a potential of I3 V measured against the calomel reference electrode shall be applied and a 10-ohm shunt resistor used, and; (I3) the test duration shall be 24 h.
- 9.4.2 Following the 24-h exposure and prior to coating evaluation, the coated steel reinforcing bar shall be given a

minimum of a 1-h cooling-off period to allow the bar to reach 20 to 30°C [68 to 86°F].

- 9.4.3 Measurements of the cathodic disbondment radius shall be obtained at 0° , 90° , 180° , and 270° . The average of the four values shall not exceed 6 mm [0.24 in.] when measured from the edge of the intentional coating defect.
- 9.4.4 Approximately even numbers of straight and bent sections of coated steel reinforcing bars shall be evaluated (approximately half of these tests shall be made on the inside radius and half on the outside radius on the bent bar sections).

10. Frequency of Tests

- 10.1 Tests for coating thickness shall be made on a minimum of two bars of straight section and two bars of bent section of each size from each production hour.
- 10.2 Tests for coating continuity shall be conducted on a minimum of two bars of each size from each production hour.
- 10.3 Tests for coating flexibility shall be made on a minimum of one bar of each bar size from each four production hours.
- 10.4 Tests for coating adhesion shall be made on a minimum of two bars from each 8-h production shift: one straight bar section and one bent bar section. If there are no bent bars being coated during the production period, a second straight bar section shall be substituted for the test.

11. Retests

- 11.1 If any test specimen for coating thickness, continuity, flexibility, or adhesion fails to meet the specified requirements, the bar from which the test specimen was taken shall be rejected and two further coated steel reinforcing bars from the same lot shall be subjected to the test or tests in which the original bar failed. If both additional coated steel reinforcing bars pass the retest, the lot from which they were taken shall be deemed to meet the requirements of this specification. If either of them fails, the lot shall be deemed not to comply with this specification.
- 11.2 If the lot does not comply with this specification, two coated steel reinforcing bars shall be selected from the lot immediately preceding and immediately following the non-compliant lot and subjected to the test or tests in which the original bars failed. If all bars pass the retests, then the lots shall be deemed to meet the requirements of this specification. If any bar fails a retest, the lot from which it originated shall be deemed not to comply with this specification.
- 11.3 In the case of a second noncompliant lot, the untested lot immediately next to it, which may be preceding or following, shall be subjected to retest. This procedure shall be repeated until a compliant lot is tested.

Note 11—In the use of the Retest provision of this specification, a "lot" is defined as the coated steel reinforcing bars that are represented by the sample which has been tested. A certain number of coated steel reinforcing bars will have been produced since the last acceptance test was performed and accepted. The steel reinforcing bars that have been coated since that last test should be divided into four equal time period groups according to when they were produced. Each lot should then be further defined as those coated steel reinforcing bars that have been produced in a given time period since the last accepted test.

12. Permissible Amount of Damaged Coating and Repair of Damaged Coating

- 12.1 Damaged coating discernible to a person with normal or corrected vision shall be repaired with patching material meeting the requirements of 5.3 in accordance with the written recommendations of the patching material manufacturer. Any rust shall be removed by suitable means before application of the patching material.
- 12.2 The maximum amount of repaired damaged coating shall not exceed 1 % of the total surface area in each 0.3-m [1-ft] length.

Note 12—If the amount of repaired damaged coating in any 0.3—m[1-ft] length of a coated bar exceeds 1 %, that section should be removed from the coated steel reinforcing bar and discarded. In patching damaged coating, care should be taken not to apply the patching material over an excessive area of the intact coating during the repair process. Too large an area of thick patching material, especially on smaller-size reinforcing bars, is likely to cause a reduction in bond strength of the bars to concrete.

12.3 Repaired areas shall have a minimum thickness of 250 μm [10 mils].

13. Inspection

13.1 The inspector representing the owner/purchaser shall have free entry at all times, while work on the contract of the purchaser is being performed, to the parts of the manufacturer's coating line that concern the manufacture of the coated steel reinforcing bar ordered. The manufacturer shall provide access to all reasonable facilities to satisfy the inspector that the coated steel reinforcing bar is being furnished in accordance with this specification. All tests and inspections shall be made at the place of manufacture prior to shipment unless otherwise specified, and shall be conducted so as not to interfere unnecessarily with the operation of the coating line. At a mutually agreed upon frequency, the purchaser or the purchaser's representative shall be permitted to take lengths of coated steel reinforcing bar from the production run for testing.

14. Rejection

14.1 Coated steel reinforcing bars represented by test specimens that do not meet the requirements of this specification shall be rejected and marked with a contrasting color paint or other suitable identification. At the manufacturer's option, the affected lot shall be replaced or, alternately, stripped of coating, recleaned, recoated, and resubmitted for acceptance testing in accordance with the requirements of this specification.

Note 13—If the coating is not to be stripped from the rejected steel reinforcing bars, the bars should be scrapped.

15. Certification

15.1 The purchaser shall be furnished, at the time of shipment, written certification that samples representing each lot of coated steel reinforcing bars have been either tested or inspected as directed in this specification and the requirements have been met. When specified in the purchase order or contract, a report of the test results shall be furnished.

16. Handling and Identification

- 16.1 Coated steel reinforcing bars shall be transported and handled with care. All systems for handling coated steel reinforcing bars shall have padded contact areas. All bundling bands shall be padded, or suitable banding used to prevent damage to the coating. All bundles of coated steel reinforcing bars shall be lifted with a strong back, spreader bar, multiple supports, or a platform bridge to prevent bar-to-bar abrasion from sags in the bundles. The coated steel reinforcing bars or bundles shall not be dropped or dragged.
- 16.2 Smaller pieces of coated steel reinforcing bars, such as stirrups and ties, shall be packaged in such a way as to minimize damage to the coating during shipping and handling.
- 16.3 If circumstances require storing coated steel reinforcing bars outdoors for more than two months, protective storage measures shall be implemented to protect the material from sunlight, salt spray, and weather exposure. If the manufacturer stores coated steel reinforcing bars outdoors without cover, the date on which the coated bars are placed outdoor shall be recorded on the identification tag of the bundled steel. Coated steel reinforcing bars stored in severe corrosive environments will require protection sooner. Coated steel reinforcing bars or bundles shall be covered with opaque polyethylene sheeting or other suitable opaque protective material. For stacked bundles, the protective covering shall be draped around the perimeter of the stack. The covering shall be secured adequately, and allow for air circulation around the bars to minimize condensation under the covering.
- 16.4 All coated steel reinforcing bars or bundles shall be stored off the ground on protective cribbing.
- 16.5 The identification of all steel reinforcing bars shall be maintained throughout the fabrication and coating process to the point of shipment.
- 16.6 Quality assurance codes that certify compliance to this specification shall be placed on all identification tags of the coated steel reinforcing bars or bundles. These codes shall include references to the date of fabrication, the date of coating, the powder coating lot designation and the quality assurance testing performed.

17. Keywords

17.1 concrete reinforcement; corrosion resistance; epoxy coating; prefabrication; steel reinforcing bars

ANNEX

(Mandatory Information)

A1. REQUIREMENTS FOR FUSION-BONDED EPOXY POWDER COATINGS FOR PREFABRICATED STEEL REINFORCING BARS

A1.1 Epoxy Powder Coatings

A1.1.1 This annex covers qualification requirements for fusion-bonded epoxy powder coatings for protecting steel reinforcing bars from corrosion.

A1.1.2 Other organic coatings may be used provided they meet the requirements of this specification.

A1.2 Test Materials

A1.2.1 A 0.5-kg [1-lb] sample of the powder coating with its generic description and its fingerprint (including the method such as infrared spectroscopy or thermal analysis) shall be submitted to the testing agency. The fingerprint and generic description shall become an integral part of the qualification test report.

A1.2.2 A sample of patching material shall be submitted to the testing agency. The product name and a description of the patching material shall be given in the test report.

A1.2.3 Test Specimens:

A1.2.3.1 The following specimens shall be submitted as a minimum for testing:

- (1) Fourteen 1.2-m [4-ft] long No. 19 [No. 6], Grade 420 [Grade 60] deformed steel reinforcing bars, with a coating thickness of 175 to 300 μ m [7 to 12 mils].
- (2) Six uncoated and uncleaned No. 19 [No. 6] steel reinforcing bars, 1.2 m [4 ft] long, from the same lot of steel as the coated bars.
- (3) Four 100 by 100 by 1.3-mm [4 by 4 by 0.05-in.] thick steel plates with center holes for Taber abrasers with a coating thickness of $250 \pm 50 \mu m$ [$10 \pm 2 \text{ mils}$].
- (4) Four 100 by 100-mm [4 by 4-in.] free films of coating material with a thickness of 175 to 225 μ m [7 to 9 mils].
- (5) Twelve coated No. 19 [No. 6] steel reinforcing bars, 0.25-m [10-in.] long, with a coating thickness of 175 to 300 μ m [7 to 12 mils]. The coated bars shall have their ends sealed with a minimum of 250 μ m [10 mils] of patching material.
- (6) Ten coated No. 19 [No. 6] steel reinforcing bars, 0.20–m [8–in.] long, with a coating thickness of 175 to 300 μm [7 to 12 mils]. The coated bars shall have their ends sealed with a minimum of 250 μm [10 mils] of patching material.

A1.2.3.2 Steel reinforcing bars with a nominal diameter within \pm 1 mm [0.04 in.] of No. 19 [No. 6] bars shall be acceptable for qualification testing.

A1.2.3.3 The coating on the bars and films tested shall be free of holes, voids, contamination, cracks, and damaged areas. The coated bars shall be checked for holidays using a 67.5-V, $80\,000$ - Ω , wet-sponge-type dc holiday detector in accordance with Test Method G 62. The total number of coating holidays found on the bar specimens tested shall be reported.

A1.2.3.4 The powder coating manufacturer shall specify the method and grade of metal surface preparation and the coating application procedures for the test specimens and for contract

production of coated steel reinforcing bars. These procedures shall be listed in the test report. Note that production-coated steel reinforcing bars shall be required to be manufactured in the same manner as the qualification bars. Therefore, it is necessary that the qualification coated steel reinforcing bars be prepared in the same manner proposed for production. Variations in the critical preparation, thermal treatment, coating procedures and cure methods known to be allowable without a compromise in quality shall also be detailed in the qualification report.

A1.3 Coating Requirements

A1.3.1 Chemical Resistance—The chemical resistance of the coating shall be evaluated in accordance with Test Method G 20 by immersing coated steel reinforcing bars in each of the following: distilled water, a 3-M aqueous solution of $CaCl_2$, a 3-M aqueous solution of NaOH, and a solution saturated with $Ca(OH)_2$. Specimens without holidays and specimens with an intentional 6-mm [0.25-in.] diameter hole drilled through the coating shall be tested. The temperature of the test solutions shall be $24 \pm 2^{\circ}C$ [75 $\pm 3.6^{\circ}F$]. The immersion shall last 45 days. The coating must not blister, soften, lose bond, or develop holidays during this period. The coating surrounding the intentionally made hole shall exhibit no undercutting during the 45-day period.

A1.3.2 Cathodic Disbondment—The effects of electrical and electrochemical stresses on the bond of the coating to steel and on the film integrity shall be assessed in elevated and ambient temperature cathodic disbondment tests. Fig. A1.1 presents the recommended equipment configuration for performing cathodic disbondment testing on steel reinforcing bars. The intentional coating defect shall be placed approximately 50 mm [2 in.] from the sealed end of the test bar centered between the longitudinal and transverse ribs. It shall be drilled just deep enough to expose a full 3–mm [0.12–in.] diameter in the steel. The test bar shall be inserted with the sealed end of the bar resting on the bottom of the test vessel and electrolyte added until 100 mm [4 in.] of the bar length is submerged.

A1.3.2.1 *Test A*—Test Method G 42 shall be followed except, (*I*) the cathode shall be a 200-mm [8-in.] long coated steel reinforcing bar; (2) the anode shall be a 150-mm [6-in.] long solid platinum electrode (1.6-mm [0.06-in.] nominal diameter) or platinized wire (3-mm [0.12-in.] nominal diameter); (*3*) a calomel reference electrode shall be used; (*4*) the electrolyte solution shall be 3 % NaCl by mass dissolved in distilled water; (*5*) the electrolyte solution temperature shall be $65 \pm 2^{\circ}$ C [$150 \pm 3.6^{\circ}$ F]; (*6*) the drilled coating defect shall be 3 mm [0.12 in.] in diameter; (*7*) a potential of -3 V measured against the calomel reference electrode shall be applied and a 10-ohm shunt resistor used; and (*8*) the test duration shall be 24 h. The tested bars shall be allowed to cool for approximately 1 ± 0.25 h prior to evaluation. Four measurements shall be



Test Bar Preparation

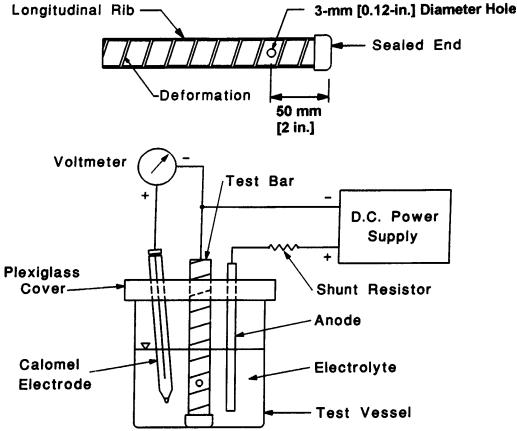


FIG. A1.1 Cathodic Disbondment Test Equipment Configuration

taken, at 0°, 90°, 180°, and 270°, and the values averaged. The average coating disbondment radius of three coated steel reinforcing bars shall not exceed 6 mm [0.24 in.] when measured from the edge of the intentional coating defect.

A1.3.2.2 Test B—Test Method G 8 shall be followed except, (1) the cathode shall be a 200-mm [8-in.] long coated steel reinforcing bar; (2) the anode shall be a 150-mm [6-in.] long solid platinum electrode (1.6-mm [0.06-in.] nominal diameter) or platinized wire (3-mm [0.12-in.] nominal diameter); (3) a calomel reference electrode shall be used; (4) the electrolyte solution shall be 3 % NaCl by mass dissolved in distilled water; (5) the electrolyte solution temperature shall be 23 \pm 2° C [74 ± 3.6°F]; (6) the drilled coating defect shall be 3 mm [0.12 in.] in diameter; (7) a potential of -1.5 V measured against the calomel reference electrode shall be applied and a 10-ohm shunt resistor used; and (8) the test duration shall be 168 h. The tested bars shall be allowed to cool for approximately 1 ± 0.25 h prior to evaluation. Four measurements shall be taken, at 0°, 90°, 180°, and 270°, and the values averaged. The average coating disbondment radius of three coated steel reinforcing bars shall not exceed 2 mm [0.08 in.] when measured from the edge of the intentional coating defect.

A1.3.3 Salt Spray Resistance—The resistance of the coating to a hot, wet corrosive environment shall be evaluated in accordance with Practice B 117 by exposing 200-mm [8-in.] long coated steel reinforcing bars containing intentional defects to $35 \pm 2^{\circ}$ C [$95 \pm 3.6^{\circ}$ F] salt spray comprised of 5 % NaCl by mass dissolved in distilled water for 800 ± 20 h. Three intentional 3-mm [0.12-in.] diameter defects shall be drilled through the coating of each test bar approximately evenly spaced along one side of the bar with the holes centered between deformations. The coated steel reinforcing bars shall be placed horizontally in the cabinet with the damage sites facing the side (90°). The tested bars shall be allowed to cool for approximately 1 ± 0.25 h prior to evaluation. Four measurements shall be taken, at 0°, 90°, 180°, and 270°, and the values averaged. The average coating disbondment radius of nine test sites on three coated steel reinforcing bars shall not exceed 3 mm [0.12 in.] when measured from the edge of the intentional coating defect.

A1.3.4 Chloride Permeability—The chloride permeability characteristics of the cured coating having a thickness of 175 to 225 μ m [7–9 mils] shall be measured on two test films and a control film at 24 \pm 2°C [75 \pm 3.6°F] for 45 days. The

permeability cells shall be of the type shown in Fig. A1.2. Films selected for testing shall be carefully handled and examined for any defects prior to installation in the cell. The cell shall consist of two glass compartments separated by a coating film sandwiched between two glass plates, each having a centered 25-mm [1-in.] hole. One compartment shall contain 175 mL [5.3 oz] of 3M NaCl and the other 115 mL [3.5 oz] of distilled water. The activity of chloride ions passing through the film shall be measured using a specific ion meter equipped with a chloride electrode and a double junction reference electrode. Activity measurements shall be converted into concentration values of mole per L [M] with a conversion diagram, constructed by plotting measured chloride ion activities versus known chloride ion concentrations. The cumulative concentration of chloride ions permeating through the film shall be less than 1 X $10^{-4}M$.

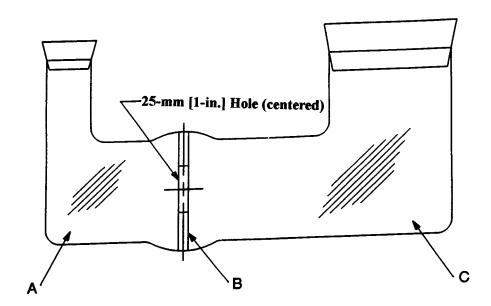
A1.3.5 Coating Flexibility—The coating shall be evaluated by bending three No. 19 [No. 6] coated steel reinforcing bars 6° (after rebound) around a 150-mm [6-in.] diameter mandrel. The bend shall be made at a uniform rate and completed within 5 s. The two longitudinal deformations shall be placed in a plane perpendicular to the mandrel radius. The specimens shall be between 20 and 30°C [68 and 86°F]. No cracking of the coating shall be discernible to a person with normal or corrected vision on the outside radius of any of the three bent bars.

A1.3.6 Relative Bond Strength in Concrete—The relative bond strength of the steel reinforcing bars to concrete shall be determined with beam-end specimens by the method described in Test Method A 944 using No. 19 [No. 6] steel reinforcing bars with a relative rib area (ratio of projected rib area normal

to the bar axis to the product of the nominal bar perimeter and the center-to-center rib spacing) between 0.075 and 0.085. The bars shall be bottom-cast and shall have a cover of 40 ± 2 mm $[1.5 \pm 0.06 \text{ in.}]$, a lead length of $13 \pm 3 \text{ mm} [0.5 \pm 0.12 \text{ in.}]$, and a bonded length of 250 \pm 5 mm [10 \pm 0.25 in.]. Test bars shall be oriented so that the longitudinal deformations and direction of rolling, relative to the direction of the applied tension, are the same for coated and uncoated bars. The test bars must be pulled in the same direction with respect to the direction of rolling. Three to six coated bar specimens and three to six uncoated bar specimens shall be tested. All steel reinforcing bars in a test group shall be from the same steel heat. The uncoated bars shall be cleaned only by lightly wiping with acetone or other suitable solvent. The mean bond strength of the coated bars shall not be less than 85 % of the mean bond strength of the uncoated bars.

A1.3.7 Abrasion Resistance—The resistance of the coating on each of three steel panels to abrasion by a Taber abraser (Test Method D 4060) or its equivalent, using CS-10 wheels and a 1-kg [2.2-lb] load per wheel, shall be such that the weight loss shall not exceed 100 mg [0.0035 oz] after 1000 cycles.

A1.3.8 *Impact Test*—The resistance of the coating to mechanical damage shall be assessed by the falling weight test. A test apparatus similar to that described in Test Method G 14 shall be used along with a 16-mm [0.63-in.] tup. Impact shall occur on the low-lying areas on the coated bars, that is, between deformations or ribs. The test shall be performed at $24 \pm 2^{\circ}\text{C}$ [75 $\pm 3.6^{\circ}\text{F}$]. With an impact of 4.5 Nm [40 in.-lb], no shattering, cracking, or bond loss of the coating shall occur except at the impact area, that is, the area permanently deformed by the tup.



Permeability Cell Components:

- A. Component containing distilled water
- B. Epoxy film sandwich between two glass plates each having a centered 25-mm [1-in.] diameter hole
- C. Component containing 3M NaCl

FIG. A1.2 Chloride Permeability Test Equipment Configuration

A1.4 Qualification Testing

A1.4.1 *Testing Agency*—Qualification tests shall be performed by an agency acceptable to the purchaser.

A1.5 Certification

A1.5.1 A report summarizing the results of all tests and bearing the signature of the testing laboratory shall be furnished to the manufacturer.

APPENDIXES

(Nonmandatory Information)

X1. APPLICATION PROCESS AND PRODUCT TEST PROCEDURES OF EPOXY COATINGS FOR STEEL REINFORCING BARS

X1.1 Scope

X1.1.1 This appendix includes recommended process and product test procedures to be included as part of a manufacturer's Quality Assurance Program for fusion-bonded epoxy coating of prefabricated steel reinforcing bars.

X1.1.2 Alternative procedures may be used provided they are acceptable to the purchaser.

X1.1.3 It is recommended that the coating application procedures and processes be audited by an independent certification program for epoxy coating applicator plants such as that provided by the Concrete Reinforcing Steel Institute, or equivalent.

X1.2 Recommended Tests

X1.2.1 This section lists the procedures recommended by this appendix.

X1.2.2 The blast media and steel reinforcing bar surface should be evaluated based on:

Screen Analysis of Blasting Abrasive (X1.4), Steel Reinforcing Bar Surface Profile Measurement (X1.5),

Steel Reinforcing Bar Surface Residue (X1.6),
Detection of Salt Contamination (X1.7) and

Detection of Mill Scale Contamination (X1.8).

X1.2.3 The epoxy coating should be evaluated based on: Porosity Rating and Interfacial Contamination (X1.9),

X1.3 Minimum Test Frequency

X1.3.1 The blast media and steel reinforcing bar surface should be evaluated a minimum of:

Screen Analysis of Blasting Abrasive

Once per shift.

Steel Reinforcing Bar Surface Profile Measurement

Three times per shift and whenever the bar size changes

Steel Reinforcing Bar Surface Residue

Three times per shift and whenever the bar size changes.

Detection of Salt Contamination

Three times per shift and whenever the bar size or steel source changes.

Detection of Mill Scale Contamination

Three times per shift and whenever the bar size changes.

X1.3.2 The epoxy coating should be evaluated a minimum of:

Porosity Rating and Interfacial Contamination

Once per shift.

X1.4 Screen Analysis of Blasting Abrasive

X1.4.1 *Scope*—This test provides a procedure to measure the particle size distribution of the abrasive.

X1.4.2 Equipment:

X1.4.2.1 *Standard Sieves*—850, 600, 425, 300, 212 μm [Nos. 20, 30, 40, 50, 70], a catch pan and lid (additional pans may be included),

X1.4.2.2 Scale or Graduated Cylinder, 100-mL [0.1-qt], and

X1.4.2.3 Funnel.

X1.4.3 Procedure:

X1.4.3.1 Obtain a representative sample of abrasive from the air wash curtain.

X1.4.3.2 Stack the screens from top to bottom from the coarsest to finest mesh with the pan at the bottom.

X1.4.3.3 Place approximately 0.45 kg [1 lb] (or 100 mL [0.1 qt]) of the sample on the top screen.

X1.4.3.4 Place the lid on the top screen and shake by hand or mechanically for 3 min.

X1.4.3.5 Measure the weight (volume) of material on each screen and in the pan. Convert these values to percentages.

X1.4.4 *Criteria*—Greater than 80 % of the abrasive should be contained on the 850, 650 and 425-µm [Nos. 20, 30, and 40-mesh] screens. Less than 3 g [0.1 oz.] of abrasive should be found in the catch pan. If these particle size distribution requirements are not met, the line should be shut down and the blast–cleaning unit checked. The test should then be repeated.

X1.5 Steel Reinforcing Bar Surface Profile Measurement

X1.5.1 *Scope*—This test measures the surface profile of abrasive blast-cleaned steel reinforcing bars.

X1.5.2 Equipment:

X1.5.2.1 Spring-loaded Dial Micrometer,

X1.5.2.2 *Replica Tape*, consisting of compressible film containing microscopic bubbles attached to a polyester film 50 µm [2 mils] thick.

X1.5.2.3 Burnishing Tool.

X1.5.3 Procedure:

- X1.5.3.1 Obtain an uncoated length (at least 1 m [3 ft]) of steel reinforcing bar that has been freshly blast-cleaned on the production line.
 - X1.5.3.2 Locate a representative surface site.
- X1.5.3.3 Peel the wax paper backing from the replica tape and apply it to the blasted surface.
- X1.5.3.4 Rub the burnishing tool over the round cut-out portion of the tape until the entire circular area has uniformly darkened.
- X1.5.3.5 Remove the replica tape from the bar and place it between the anvils of the spring micrometre. Subtract 50 μ m [2 mils] from the gage reading to compensate for the thickness of the plastic film. The gage reading is the average peak to valley height of the blasted surface.
- X1.5.3.6 Perform the procedure on two other areas of the test bar.
- X1.5.4 *Criteria*—The suitable average blast profile roughness depth range is specified in 7.1.2. If this anchor profile range is not found, the line should be shut down and the blast-cleaning unit checked. The test should then be repeated.

X1.6 Steel Reinforcing Bar Surface Residue

X1.6.1 *Scope*—This test measures the amount of visible residue left on the steel reinforcing bar surface by the blast-cleaning process.

X1.6.2 Equipment:

X1.6.2.1 White Adhesive Tape (3M #471 plastic tape or equivalent), 6-mm [0.25-in.] wide.

X1.6.2.2 Visual Contamination Standards (see Fig. X1.1).

X1.6.2.3 Magnifying Glass or Microscope, 30×.

X1.6.2.4 Marker Pen,

X1.6.2.5 Utility Knife, and

X1.6.2.6 Burnishing Tool.

X1.6.3 *Procedure*:

- X1.6.3.1 Obtain an uncoated length (at least 1 m [3 ft]) of steel reinforcing bar that has been freshly blast-cleaned on the production line.
- X1.6.3.2 Approximately 300 mm [12 in.] from one end of the bar, place a mark between the deformations with the marker pen. Between the next set of deformations, apply a piece of the adhesive tape. Skip the next two valleys and place a second piece of adhesive tape between the next set of deformations. In the next valley, place a second mark with the marker pen.

X1.6.3.3 Rub the tapes lightly with a burnishing tool.

X1.6.3.4 Remove the tapes.

X1.6.3.5 Examine the darkest area on the tapes with a $30 \times$ magnifying glass or microscope and compare them to the visual contamination standards to determine the percentage of backside contamination.

X1.6.3.6 Approximately 300 mm [12 in.] from the other end of the bar, and on the other side from the first test site, apply another piece of the adhesive tape. Repeat X1.6.3.3-X1.6.3.5

X1.6.3.7 Bend the bar 180° around a 6 *d* mandrel with the marked area on the outside radius of the bend. Take care not to contaminate the marked area before and after the bend.

X1.6.3.8 In the marked area, apply a fresh piece of the adhesive tape between the set of deformations next to the first mark. Skip the next two valleys and place a second piece of adhesive tape between the next set of deformations. The next

valley should contain the second mark from the marker pen. The new tapes should be located in the same positions as the first set of tapes applied before bending.

X1.6.3.9 Repeat steps X1.6.3.3-X1.6.3.5.

X1.6.4 *Criteria*—The percent contamination on the straight and bent bar samples should not exceed 30 %. If the percent contamination is in excess of this value, the line should be shut down and the blast-cleaning unit checked. The test should then be repeated.

X1.7 Detection of Salt Contamination

X1.7.1 *Scope*—This test is used to detect chloride that may be present on the steel reinforcing bar surface after blast cleaning and in the abrasive media.

X1.7.2 Equipment:

X1.7.2.1 Potassium Ferricyanide-coated Paper Strips,

X1.7.2.2 Distilled Water,

X1.7.2.3 Plastic Bags,

X1.7.2.4 Plastic Spray Bottle,

X1.7.2.5 Rubber Gloves,

X1.7.2.6 Tweezers, and

X1.7.2.7 Chloride Visual Standard (see Fig. X1.2).

X1.7.3 Procedure:

X1.7.3.1 Upon receipt, store the potassium ferricyanide-coated strips in a sealed plastic bag. Protect the strips from exposure to light. The test paper should have a yellow color (no blue color should be observed). Even with proper care, the test strips deteriorate over time and should be replaced within the time recommended by the test paper manufacturer.

X1.7.3.2 Obtain an uncoated length of at least 1 m [3 ft] of steel reinforcing bar that has been freshly blast-cleaned on the production line.

X1.7.3.3 Wet the test paper with distilled water until the paper is saturated. Allow any excess water to drip off.

X1.7.3.4 Lightly press the test paper onto the reinforcing steel bar surface and maintain contact for 30 s. Remove the paper, turn it over and observe for color change. The presence of soluble ferrous chloride will be indicated by a blue color.

X1.7.3.5 When testing the abrasive media, sprinkle the abrasive on the wet test paper until it is covered. Leave the abrasive on the paper for 30 s.

X1.7.3.6 Avoid contact of the test paper with the skin.

X1.7.3.7 Compare the test strip to the Chloride Visual Standard to estimate the chloride concentration on the steel.

X1.7.3.8 Perform the procedure on two other areas of the test bar.

X1.7.4 *Criteria*—If chloride is found to be present on the blasted steel reinforcing bar or in the abrasive media, repeat the test. If chloride is still observed in the new sample, shut the line down until the source of the contamination is found and corrected. The test should then be repeated.

X1.8 Detection of Mill Scale Contamination

X1.8.1 *Scope*—This test is used to determine the cleanliness of the blast–cleaned steel reinforcing bar surface when examined for mill scale.

X1.8.2 Equipment:

X1.8.2.1 Copper Sulfate (anhydrous),

X1.8.2.2 Distilled Water,

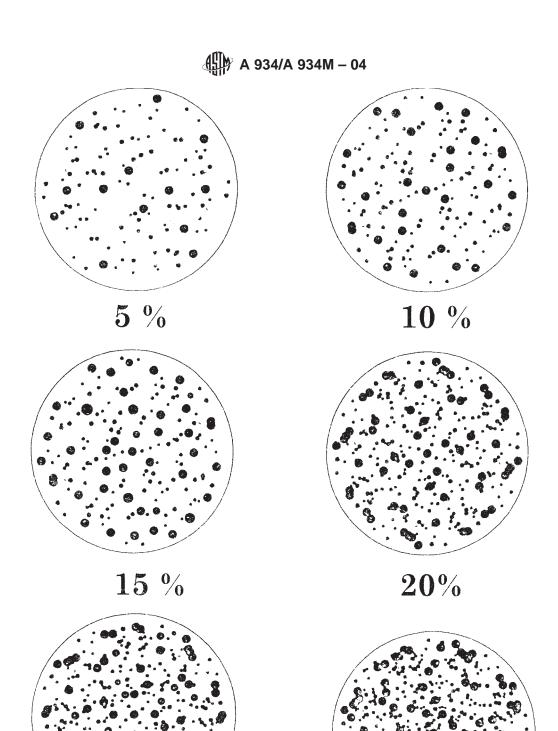


FIG. X1.1 Backside Contamination Visual Standard

X1.8.2.3 *Clean Bottle*, for mixing the solution and an eyedropper,

25 %

X1.8.2.4 A Magnifying Glass or Microscope, 30× and X1.8.2.5 Copper Sulfate Visual Standard (see Fig. X1.3). X1.8.3 Procedure:

X1.8.3.1 Dissolve 5 % by mass copper sulfate in distilled water.

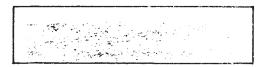
X1.8.3.2 Obtain an uncoated length of at least 1 m [3 ft] of steel reinforcing bar that has been freshly blast-cleaned on the production line.



 $1 \times 10^{-6} \text{ g/cm}^2$

 $5 \times 10^{-6} \text{ g/cm}^2$





 $10 \times 10^{-6} \text{ g/cm}^2$

 $15 \times 10^{-6} \text{ g/cm}^2$



$25 \times 10^{-6} \text{ g/cm}^2$

FIG. X1.2 Detection of Chlorides-paper Test Visual Standard

X1.8.3.3 Apply a few drops of copper sulfate solution to the blasted surface and allow it to sit for 1 min.

X1.8.3.4 Clean steel reacts with copper sulfate with the surface iron replacing the copper in solution thus plating out the copper. The copper sulfate will not react with mill scale, dirt, or rubber.

X1.8.3.5 Use a $30 \times$ magnifying glass or microscope to estimate the amount of mill scale and other nonreactive contaminants by comparing the surface to the Copper Sulfate Visual Standard.

X1.8.3.6 As a minimum, this test should be conducted once on opposite sides of the bar (a minimum of two tests per bar).

X1.8.4 *Criteria*—If the percentage of mill scale is found to be in excess of 5 % of the steel surface, the line should be shut down and the blast–cleaning unit checked. The test should then be repeated.

X1.9 Porosity Rating and Interfacial Contamination

X1.9.1 *Scope*—This porosity rating method provides a numerical value for the level of porosity in a coating. The interfacial contamination test also provides an indication of the amount of visible residue left on the substrate from the abrasive cleaning process. These procedures are combined as they utilize the same test specimens.

45 A 934/A 934M – 04

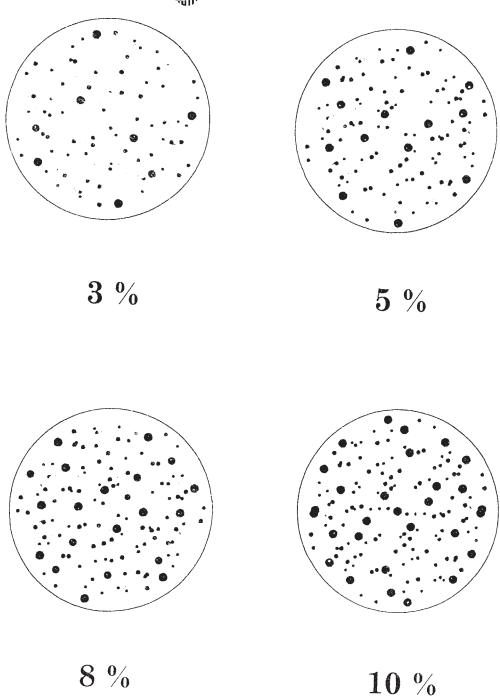


FIG. X1.3 Copper Sulfate Visual Standard

- X1.9.2 Equipment:
- X1.9.2.1 Magnifying Glass or Microscope, 30×,
- X1.9.2.2 Bend Equipment,
- X1.9.2.3 Freezer or Dry Ice,
- X1.9.2.4 Copper Sulfate Solution, 5 % by mass,
- X1.9.2.5 Porosity Charts A and B (see Fig. X1.4), and,
- X1.9.2.6 Interfacial Contamination Comparison Chart-(Fig. X1.1 can be used).
- X1.9.3 Procedure:
- X1.9.3.1 Obtain a coated length (at least 1 m [3 ft]) of steel reinforcing bar that has been freshly coated on the production line.

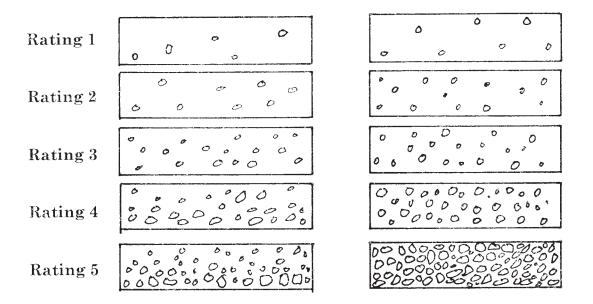
X1.9.3.2 Chill the sample with dry ice or a suitable freezer (to -40° C [-40° F] or below) for a minimum of 30 min. Bend the specimen approximately 30° over a bar pin mandrel with a radius of 13 mm [0.5 in.] or as appropriate to crack the coating. A large enough chip sample should be removed to give a good representation of the steel surface (at least 100 mm² [0.16 in.²]).

X1.9.3.3 *Porosity Rating*—Examine the cross-section of the coating under a magnification of $30 \times$ and determine the porosity level based on Porosity Chart A. If the cross-sectional porosity rating is 2 or less, no interfacial test is required.



Porosity Chart A: Cross-sectional porosity

Porosity Chart B: Porosity at plane of cohesive failure



Example of coating chip:

Check interfacial porosity in these areas.

Check backside contamination in these areas.

Check cross sectional porosity in these areas.

FIG. X1.4 Porosity Charts A and B

X1.9.3.4 If the rating is 3 or greater, or if the bulk of the void areas are within 20 % of the coating nearest the substrate, a sharp razor blade should be used to remove a thin layer from the underside of the coating exposing its degree of porosity. The interfacial porosity can then be estimated using Porosity Chart B

X1.9.3.5 Interfacial Contamination Rating—Observe the backside (the side originally against the steel surface) of the coating under a magnification of 30× and estimate the amount of non-coating material as a percentage of total viewed area using the Interfacial Contamination Comparison Chart. Record this value as "Percent Contaminated Area."

X1.9.3.6 Dip the coating sample into copper sulfate solution for 10 s and again observe the backside of the coating under magnification. The contamination that is primarily steel will

change to a copper color. Record this value as "Percent Contaminated Area that is Steel."

X1.9.4 *Criteria*—If a rating of 3 or greater is determined on the interfacial porosity measurement or the non-steel contamination level is greater than 30 % on the interfacial contamination test, the line should be shut down until the source of the porosity/contamination is found and corrected. The test should then be repeated.

X1.10 Cathodic Disbondment Testing

X1.10.1 Procedure and acceptance criteria as outlined in A1.3.2 and A1.3.2.2.

X1.11 Salt Spray Testing

X1.11.1 Procedure and acceptance criteria as outlined in A1.3.3.

X2. GUIDELINES FOR JOB-SITE PRACTICES

X2.1 This specification is a product standard. Requirements for fusion-bonded epoxy coated prefabricated steel reinforcing bars from the point of shipment to the job-site and subsequent practices at the job-site are not delineated in this product standard.

X2.2 The American Concrete Institute promulgates "Specifications for Structural Concrete" (ACI 301). Standard Specifications ACI 301 is intended to be used in its entirety in the project specifications. An architect-engineer may cite Standard Specifications ACI 301 in the project specifications for any cast-in-place concrete construction project. Standard Specifications ACI 301 include provisions for epoxy-coated steel reinforcing bars.

X2.3 The project specifications should prescribe requirements for the coated steel reinforcing bars from the point of shipment to the job-site and subsequent practices at the job-site. In the absence of these requirements in the project specifications, the following guidelines for job-site practices are recommended.

X2.3.1 When handling coated steel reinforcing bars, care should be exercised to avoid bundle-to-bundle or bar-to-bar abrasion.

X2.3.2 Equipment for handling coated steel reinforcing bars should have protected contact areas.

X2.3.3 Coated steel reinforcing bars should be off-loaded as close as possible to their points of placement or under the crane so that the bars can be hoisted to the area of placement to minimize rehandling.

X2.3.4 Coated steel reinforcing bars should be stored off the ground on protective cribbing, and timbers placed between bundles when stacking is necessary. Space the supports sufficiently close to prevent sags in the bundles.

X2.3.5 Coated and uncoated steel reinforcing bars should be stored separately.

X2.3.6 Long-term storage should be minimized and work stoppages phased to suit construction progress.

X2.3.7 If circumstances require storing coated steel reinforcement outdoors for more than two months, protective storage measures should be implemented to protect the material from sunlight, salt spray and weather exposure. If the coated steel reinforcing bars are stored outdoors without protective covering, it is recommended that the date on which the coated bars are placed outdoors be recorded on the identification tag on the bundled steel. Coated steel reinforcing bars stored in corrosive environments may require protection sooner. Coated steel reinforcing bars or bundles should be covered with opaque polyethylene sheeting or other suitable protective material. For stacked bundles, the protective covering should be draped around the perimeter of the stack. The covering should be secured adequately, and allow for air circulation around the bars to minimize condensation under the covering.

X2.3.8 When the extent of damaged coating exceeds 2 % of the surface area of the coated steel reinforcing bar in any 0.3-m [1-ft] length, the coated bar should be rejected.

X2.3.9 When the extent of the damage does not exceed 2 % of the surface area in any 0.3-m [1-ft] length, all damaged coating discernible to a person with normal or corrected vision should be repaired with patching material.

Note X2.1—If the amount of repaired damaged coating in any 0.3–m [1–ft] length of a coated bar exceeds 2 %, that section should be removed from the coated steel reinforcing bar and discarded. In patching damaged coating, care should be taken not to apply the patching material over an excessive area of the intact coating during the repair process. Too large an area of thick patching material, especially on smaller-size reinforcing bars, is likely to cause a reduction in bond strength of the bars to concrete.

X2.3.10 Coated steel reinforcing bars should not be flame cut.

X2.3.11 Placed coated steel reinforcing bars should be inspected for coating damage prior to placing concrete. Where damage exists, it should be repaired with patching material complying with this specification.

X2.3.12 Patching material should be applied in strict accordance with the written instructions furnished by the patching material manufacturer. Prior to application of the patching

material, rust should be removed from the damaged areas by suitable means. The patching material should be allowed to cure before placing concrete over the coated steel reinforcing bars.

X2.3.13 When placing coated steel reinforcing bars, all wire bar supports, spacers, and tying wire should be coated with dielectric material, for example, an epoxy-coated or plastic-coated material compatible with concrete.

X2.3.14 After placing, walking on coated steel reinforcing bars should be minimized. The placement of mobile equipment should be planned to avoid damage to the coated bars.

X2.3.15 When immersion-type vibrators are used to consolidate concrete around epoxy-coated steel reinforcing bars, the vibrators should be equipped with nonmetallic, resilient vibrator heads.

X3. PERMISSIBLE INCREASED COATING THICKNESS ON BENT SECTIONS OF BARS

X3.1 In practice, coating thicknesses greater than 300 µm [12 mils] are prevalent on the inside and outside bend regions of prefabricated bars. The greater coating thicknesses in the bend areas occur due to powder depositing on surfaces of bars facing in an upward position during powder application. The greater coating thicknesses may occur on the outside radius, the inside radius, or both radii depending on the geometry of the bar and its position in the spray booth.

X3.2 Section 9.1.1 has been revised to reflect industry practice. The maximum coating thickness on straight sections of bars is limited to 300 μ m [12 mils]. On bent sections of bars, the maximum permissible coating thickness is 405 μ m [16 mils].

X3.3 The maximum permissible coating thickness in the bend areas may have an adverse effect on the anchorage capacity of bent bars embedded in concrete. For example, if the coating thickness on the inside radius of a standard end hook is $405~\mu m$ [16 mils], a longer embedment length may be required

to fully develop the bar than the length prescribed by the current design standards. On the other hand, it is unlikely that a 405-µm [16-mil] coating thickness on the outside radius of a standard end hook would have a detrimental effect on anchorage capacity. The anchorage capacity of coated bars with standard end hooks, where the end hooks engage other reinforcing bars, should not be affected.

 $X3.4\,$ Subcommittee A01.05 is committed to play a meaningful role in the resolution of this dilemma. The subcommittee will strive to foster research with the objective of determining whether the maximum permissible coating thickness of 405 μm [16 mils] in the bend areas of bent bars causes a reduction in anchorage capacity, and if so, to quantify the reduction.

X3.5 Until the high-priority research has been completed and its findings are disseminated, designers are urged to carefully evaluate those conditions where less than the anticipated anchorage capacity of bent bars may result.

SUMMARY OF CHANGES

Committee A01 has identified the location of the following changes to this standard since A 934/A 934M-03 that may impact the use of this standard. (Approved March 1, 2004.)

(1) Revised section 9.1.3.

Committee A01 has identified the location of the following changes to this standard since A 934/A 934M-01 that may impact the use of this standard. (Approved June 10, 2003.)

- (1) Revised section X.2.3.15.
- (2) Revised section 9.2.2.
- (3) Deleted Test Method G12 from Section 2.1 and added

Specification SSPC-PA2 to Section 2.3.

(4) Revised Sections 2.4 and 9.1, and added Appendix X3.

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